# Study on Fresh And Hardened Properties of High Strength Self Compacting Concrete With Metakaolin And Microsilica As Mineral Admixture (M70 Grade)

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Abstract: This experimental study demonstrates that metakaolin (MK) & Micro Silica(MS) can be successfully used as an admixture in the preparation of self-compacting concrete (HSSCC). In order to prepare suitable mix proportions for different grades of MK based HSSCC, investigations were undertaken replacing cement with 0%, 10%, 20%, 30%, 40% of MK As per the European guidelines for Self-compacting concrete, slump flow test, V-funnel test and L-box test have been carried out on fresh properties of MK based HSSCC. The compressive strength, split tensile strength and flexural strength of the specimens have been analyzed for 7-days and 28 days curing. From the study it is observed that workability decreases with increase in Metakaolin. Whereas mechanical properties such as Compressive strength, Split tensile strength and Flexural strength test increase with increase in metakaolin up to 20% and decreases from 30% to 40%. As metakaolin increases from 0% to 40%, slump value decreases by 6.42%%, V funnel at T5 minutes value increases by 23.04% and L Box value decreases by 16.32 % and U Box value increases by 33%. The fresh concrete test results revealed that by substituting different percentages of MK in SCC, satisfactory workability and rheological properties can be achieved, by using viscosity modifying agent. In general, it seems that MK can be considered as suitable replacement regarding to the economic efficiency, fresh and hardened properties of MK based HSSCC. Non Destructive Test is also conducted to assess the quality of concrete in the hardened state. It was observed that quality of concrete is excellent for metakaolin replacement of 10%&20% and good for 30% replacement. Whereas quality of concrete is satisfactory for a 40% replacement.

Keywords: Self Compacting concrete, Metakaolin, Micro silica NDT, Fresh and Hardened properties.

## I. Introduction

The use of sustainable technologies such as supplementary cementitious materials (SCMs) is expected to positively affect the performance of concrete mixtures. Self Compacting Concrete (SCC) is an innovative development of conventional concrete, which requires high binder content to increase segregation resistance. SCC is a high flowable concrete, which can be placed and compacted without any vibration in complex or dense reinforced formworks. In order to achieve such behavior, the main requirements of fresh SCC are filling ability, passing ability and very high segregation resistance. The first two properties can be achieved by using a super plasticizer admixture. To secure stability/cohesion of the mix, a large quantity of powder materials and/or viscosity-modifying admixture (VMA) is required. The hardened SCC is dense, homogeneous and has the same engineering properties and durability as that of traditional vibrated concrete. The use of SCC eliminates the need for compaction thereby saves time, reduces labour costs and conserves energy. Furthermore use of SCC enhances surface finish characteristics. According to EFNARC, the term powder is defined for materials of particle size smaller than 0.125 mm which includes fraction of aggregate, additions and cement. Portland cement is a highly energy-intensive product. On the other hand, CO2 emmission are produced by the cement industry. In addition, some disadvantages in the properties of concrete have been reported as the cement content exceeded a specified value. To minimize these negative effects, the requirement to increase powder content in SCC is usually met by the use of additions. For this purpose, substantial studies have been performed on the usage of different additions for partially replacement of cement in SCC or self-compacted mortar such as marble powder, limestone powder, basalt powder, fly ash and slag. For instance, for fast track construction, early age compressive strength reduction may be unacceptable. The utilization of high pozzolanic activity materials seems to be an efficient choice to overcome this difficulty.

Metakaolin (MK) can also be considered as mineral admixture in the production of SCC. MK (commercially available since the mid-1990s) is a thermally activated alumino-silicate material mostly produced by calcination of kaolin clay at temperature ranging from 700°C to 850 °C without production of CO2. MK processing involves lower temperatures than Portland cement which may yield a lower cost on MK production. But, due to the low production of MK, the price will be raised up. Nonetheless, the usage of MK in concrete can be reasonable due to its environmental benefits and positive effect on the both short and long terms

strength of concrete.In this respect, it has been reported that the use of MK in concrete can increase the compressive strength of mixtures especially during early ages of hydration. Poon et al. showed that at early ages, the higher pozzolanic activity of MK results in a higher rate of strength development and pore structure refinement when compared to silica fume -or fly ash- blended cement pastes. However, at the same level of replacement, MK concrete had similar strength after 28 days with respect to silica fume concrete. Kim et al. measured the concrete strength with Korean MK and suggested that 10% replacement of MK is an appropriate replacement. However, different aspects of normal concrete containing

MK have been reported in literature but to the authors' knowledge, the performance of MK in SCC is not well documented, particularly over a wide range of grades. In particular, the effects of MK as a high surface area mineral addition on the workability as well as mechanical properties of SCC need to be fully recognized. So, the present study is an effort to characterize the fresh and hardened properties of SCC containing MK.

For this purpose, several tests concerning slump flow, V-funnel and L-box were conducted to assess the workability of the matrix. Furthermore, hardened properties were evaluated by compressive strength, splitting tensile strength and flexural strength.

## **II. Experimental Plan**

## 2.1. General

The scope of present work is to study on fresh and hardened properties of Self Compacting Concrete with metakaolin & microsilica as mineral admixture (M70 Grade). The work focused on replacement of metakaolin 10%,20%,30%,40% & micro silica 8%, with a Packing factor of 1.10. The Concrete mixes contains different proportions of Metakaolin & cement and constant proportions of waterbinder, microsilica, Coarse aggregate and Fine aggregate for constant water-cement ratios. The mix proportions are obtained on the basis of NAN-SU mix design. Non Destructive Test is also conducted to assess the quality of concrete in the hardened state.

### 2.2. Materials

The 53 grade ordinary Portland cement and MK (brought from 20 microns Ltd, Mumbai) were used as binder materials in the production of concrete mixes. The fine aggregate was natural river sand. Limestone gravel a nominal maximum size of 10 mm was used as coarse aggregate. Poly carboxylic ether based admixture namely Glenium B233 (brought from BASF) was used to enhance the flow ability of the mixtures. The physical properties of all the materials are shown in following tables.

S. No	Property	Test Results
1.	Normal Consistency	30%
2.	Specific gravity	2.9
3.	Initial setting time Final setting time	99 minutes 207 minutes 207 minutes
4.	Fineness	1.3%
5.	Soundness	2 mm

Table: 1 Physical properties of Ordinary Portland cement

Table: 2	<b>2</b> Physical	properties of	of Fine aggregate and	Coarse aggregate
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S.	Property	Fine	Coarse
No		Aggregate	Aggregate
1.	Specific gravity	2.42	2.6
2.	Bulk Density	1545.69	1400 kg/m <sup>3</sup>
	Loose	kg/m <sup>3</sup>	
3.	Bulking	4% w c	
4.	Flakiness Index		8%
5.	Elongation Index		0%
6.	Fineness Modulus	2.64	

S.No	Description of physical properties	Units	Results
1	Color		1Close To Std
2	Appearance		1 OFF white Powder
3	Bulk Density	Gm/liter	356
4	Oil Absorption	Gm/100gm	
5	Moisture (EX-Work)	%	0.22
6	PH (10% A2 Slurry)		6.22
7	RESIDUE on 325 Mesh	%	0.13
8	PSD –D(50)- 50% particles	μ	1.68
9	Specific gravity		2.63

S. No	Property	Results
1.	Form or state	Liquid
2.	Color	Brown
3.	Specific Gravity Final setting time	1.09 at 30°C

Table: 4 Properties of Super plasticize
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## 2.3. Mix Proportions for M70 SCC (metakaolin variable)

Mix Components	Concrete Mixes					
	MIX 1	MIX 2	MIX 3	MIX 4	MIX 5	
	0%	10%	20%	30%	40%	
	Qty.	Qty.	Qty.	Qty.	Qty.	
CEMENT	574.02	517.00	459.216	401.81	344.41	
C.A	790.94	790.94	790.94	790.94	790.94	
F.A	829.4	829.4	829.4	829.4	829.4	
WATER	173.26	173.26	173.26	173.26	173.26	
METAKAOIN	52.400	109.802	167.2	224.608	282.08	
S.P	11.28	11.28	11.28	11.28	11.28	
MICRO SILICA	40.18	40.18	40.18	40.18	40.18	
V.M.A	1.722	1.722	1.722	1.722	1.722	

#### Table: 5 Mix Proportions for M70 SCC

#### 2.4. Tests on Fresh Concrete

The test mentioned in Table 6 was conducted to assess whether the mixes meet the workability requirements of SCC. The results of tests conducted on fresh concrete are given in Table 7.

Table: 6 N	Aethods for	testing	workable	properties	of SCC
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S.No	METHOD	PROPERTY
1	Slump flow by Abrams cone	Filling ability
2	T <sub>50</sub> Slump flow	Filling ability
3	V-Funnel	Filling ability
4	V-Funnel at T5 minutes	Segregation resistance
5	L-Box test	Passing ability
6	U-Box test	Passing ability

Workability Tests	Concrete I	Concrete Mixes			
	MIX 1	MIX 2	MIX 3	MIX 4	MIX 5
	0%	10%	20%	30%	40%
Slump flow (mm)	700X7	695X69	685X68	670X670	655X655
	00	5	5		
T 500(sec)	4.12	4.2	4.45	4.75	4.98
V-funnel(sec)	9.12	10.65	11.35	11.5	11.8
V-funnel T5min(sec)	12.15	13.20	14.35	14.65	14.95
L-box(h2/h1)	0.98	0.95	0.92	0.85	0.82
U-box (mm)	3	4	5	6	7

#### Table: 7 Fresh properties of M70 SCC

 Table: 8 Acceptance criteria for SCC (as per EFNARC)

Method	Unit	Typical range of values	
		Min	Max
Slump flow by Abrams cone	mm	650	800
T <sub>50</sub> slump-flow	Sec	2	5
V-funnel	Sec	6	12
V-funnel at T5 minutes	Sec	6	15
L-Box	$H_2/H_1$	0.8	1.0
U-Box	$(h_2-h_1)mm$	0	30

From Table7 and 8, it is observed that all mix proportions satisfy the workability requirements of Self Compacting Concrete.

#### 2.5. Compressive Strength Tests

Compressive strength tests were conducted on 100 mm size cubes of SCC in a compression testing machine at 7 and 28 days. The results are given in Table 9 and plotted in Fig.1.

Table. 7 Compressive Strength Results			
Type of mix	Compressive Strength		
	(N/mm2)		
	7days	28days	
MIX 1 (0%)	54.95	78.5	
MIX 2 (10%)	57.715	82.45	
MIX 3 (20%)	58.177	83.11	
MIX 4 (30%)	54.93	78.48	
MIX 5 (40%)	54.544	77.92	





Figure 1: Metakaolin Vs Compressive strength

From Table 9 and Fig.1, it is observed that as the Metakaolin increases compressive strength decreases this is due to increase in powder content.

## 2.6. Split Tensile Strength Tests

Splitting tensile strength tests were conducted on cylindrical specimens of 150 mm diameter and 300 mm height at 7 and 28 days in accordance with BIS specifications and procedures. The results are given in Table 10.

Table: 10 Split Tensile Strength Results			
Split Tensile Strength (N/mm2)			
7days	28days		
3.65	4.04		
3.85	4.242		
3.965	4.284		
3.765	4.069		
3.38	3.743		
	Split Tensile Str           7days           3.65           3.85           3.965           3.765		



Figure 2: Metakaolin Vs Split Tensile strength

From Table 10 and Fig.2, it is observed that as the water cement ratio increases split tensile strength decreases this is due to increase in powder content.

## 2.7. Flexural Strength Tests

Flexural tests were conducted on beams of size of 100 mm x 100mm X 500 mm subjected to two point loading at 28 days in UTM and the results were given in Table 11. These results are plotted in Fig.3.

Table: 11 Flexural Strength Results			
Type of mix	Flexural Strength (N/mm2)		
	7days	28 days	
MIX 1 (0%)	5.18	6.201	
MIX 2 (10%)	5.32	6.352	
MIX 3 (20%)	5.33	6.381	
MIX 4 (30%)	5.19	6.2	
MIX 5 (40%)	5.16	6.179	



Figure 3: Water cement Ratio Vs Flexural strength

## 2.8. Non Destructive Tests

The main objective of the present experimental investigations is to assess the quality, structural integrity and estimated compressive strength of metakaolin incorporated self compacting concrete of grade M70 using Rebound hammer and Ultrasonic pulse velocity measurements.

## 2.9. Rebound Hammer Test

Rebound hammer is the oldest technique used to assess the compressive strength of concrete indirectly and also to compare the various parts of structure. Schmidt rebound hammer is the instrument for this test. Schmidt rebound hammer shown in Fig1 is a simple, handy tool, which can be used to provide a convenient and rapid indication of the compressive strength of concrete. It consists of a spring controlled mass that slides on a plunger within a tubular housing. It works on the principle that the rebound of an elastic mass depends on the hardness of the surface against which the mass impinges.



Figure 3 : Rebound hammer used

Table: 1	12 Quality	of concrete based on Average Rebound H	lammer
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Average rebound number	Quality of concrete
> 40	Very good hard layer
30 to 40	Good layer
20 to 30	Fair
< 20	Poor

## 2.9. Ultrasonic Pulse Velocity Test

Three reading of ultrasonic pulse velocity (USPV) were obtained for each cube. The cubes were then give a load of 7 N/mm2 (as specified by the IS code 13311) in the compression testing machine and the USPV were obtained. The cubes were then loaded upto their ultimate stress and the breaking load was obtained. The method starts with the determination of the time required for a pulse vibrations at an ultrasonic frequency to travel through a concrete. Once the velocity is determined, an idea about quality, uniformity, condition and strength of the concrete tested can be attained. In the test, the time the pulses take to travel through concrete is recorded. Then the velocity is calculated as:

## V = L/T

#### Where

V= Pulse Velocity, L = travel time in meters and

T = effective time in seconds



Figure 10: Ultrasonic Pulse Velocity used

Pulse Velocity	Quality of concrete
> 4.0 km/s	Very good to excellent
3.5 to 4.0 km/s	Good to very good, slightly porosity may exist
3.0 to 3.5 km/s	Satisfactory but loss of integrity may exist
< 3.0 km/s	Poor and loss of integrity exist

Table: 14	Non-destructive	test results o	f M70	grade SCC
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%(MK)	Combined Rebound hammer and Ultrasonic Pulse Velocity Methods Results				
	M70 Concrete, Mpa				
	MeanMean PulseCompressiveQuality ofReboundVelocityStrengthConcrete				
	Value	km/sec	N/mm2		
0%	31.5	3.6	79.8	Good	
10%	42.4	4.03	83.25	Excellent	
20%	44.3	4.12	82.1	Excellent	
30%	32.1	3.5	78.5	Good	
40%	28.5	3.4	78	satisfactory	

From the above results it was observed that quality if concrete is excellent for replacement for 10% & 20% and good for replacement for 30%. whereas quality of concrete is satisfactory for 40% v replacement.

## **III. Discussion Of Test Results**

#### 3.1. Discussion on proportions of mix

The mix proportions of SCC for M70 grade of concrete obtained "on the basis of Nan Su method" is shown in the **Table 5** for different percentage of metakaolin Viz. 0%,10%,20%,30% and 40% by keeping PF as 1.10. It was noted that, as the metakaolin % increases from 0% to 40%, the powder content increases and the aggregate content remains the same. For the mix proportions obtained, **Tables 7 and 9,10,11** shows the details of fresh properties and mechanical properties for various metakaolin percentages.

### 3.2. Effect of metakaolin on fresh properties

The workability decreased with the increase in the metakaolin. This is because of increase in metakaolin .

As the metakaolin increases from 0% to 10% Slump value decreases by 0.714%, T500 value increased by 1.94%, V funnel value increased by 16.77%, V funnel at T5 minutes increased by 5%,L box value decreased by 3.06%, U box value increases by 33.33%.As the metakaolin increases from 0% to20% Slump value decreases by 2.142%, T500 value increased by 8.09%, V funnel value increased by 24.45%, V funnel at T5 minutes increase by 18.1%, L box value decreased by 16.122%, U box value increases by 66.67%.As the metakaolin increases from 0% to 30% Slump value decreases by 4.285%, T500 value increases by 15.29%, V funnel value increases by 26.096%, V funnel at T5 minutes increases by 20.57%,L box value increases by 13.26%, U box value increases by 100%.

As the metakaolin increases from 10% to 40% Slump value decreases by 6.42 %, T500 value increases by 18.93%, V funnel value increases by 29.385%, V funnel at T5 minutes increases by 23.04 %,L box value increases by 16.32%, U box value increases by 133%. From the above results we observed that the workability decreases gradually as the percentage of metakaolin increases from 0% to 40%. Further it is observed that the change in the percentage variation of flow values increases as the percentage of metakaolin increases from 10% to 20%, 20% to 30%, and 30% to 40% the reason for this is the powder content increases as the water cement ratio is constant.

### 3.3. Effect of metakaolin on hardened properties

The results of the mechanical properties such as compressive strength, split tensile strength and flexural strength are noted by testing the specimens "as per Indian standard test procedures (as per IS: 516)" are presented in **Table 9,10,11**. As the metakaolin increases from 0% to 10% Compressive Strength increases by 5.03 %, Split Tensile Strength increases by 5.47 %, Flexural Strength increases by 2.70 %.

As the metakaolin increases from 0% to 20% Compressive Strength increases by 5.87%, Split Tensile Strength increases by 2.89 %, Flexural Strength increases by 8.63 %. As the metakaolin increases from 0% to 30% Compressive Strength decreases by 0.036 %, Split Tensile Strength decreases by 3.15 %, Flexural Strength decreases by 0.193% As the metakaolin increases from 0% to 40% Compressive Strength decreases by 0.73%, Split Tensile Strength decreases by 7.39 %, Flexural Strength decreases by 0.386 %. From the above results it is observed that the strength increases gradually as percentage of metakaolin increases from 0% to 20%. And strength decreases gradually as percentage of matakaolin increases as the percentage of metakaolin increases from 10% to 20% and decreases from 30% to 40%, the reason for this is the powder content increases as the cement ratio is constant as per the Nan-Su mix design calculations..

#### 3.4. Non Destructive Evaluation

Mean Rebound values and Mean Ultrasonic Pulse Velocities are measured to understand the quality, integrity and strength of Self Compacting Concrete. From the results it is observed that the surface hardness and quality of concrete are Excellent for percentage 10% and 20%. For replacement 30% the quality of concrete is good. For ratio 40% the quality of concrete is satisfactory.

#### **IV. Conclusions**

As the replacement rises from 0% to 40%,

- The powder content increases and the aggregate content remains the same.
- The workability decreases gradually as the percentage increases from 0% to 40%.
- The change in the percentage variation of flow values decresses as the percentage of metakaolin increases from 10% to 20% and 30% to 40%, the reason for this is the powder content increases as the water cement ratio is constant.
- The compressive strength, split tensile strength and flexural strength increased with the increase in the percentage of metakaolin from 0% to 20% & decreases from 30% to 40%.
- The percentage variation of strength values increases as the percentage of metakaolin increases from 0% to 20% and decreases from 30% to 40% the reason for this is the powder content increases as the water cement ratio is constant. It leads to decrease the binding capacity of the concrete mix.
- The optimum percentage of metakaolin is taken as 20% for w/c=0.27.
- The presence of metakaolin improved both early ages and long term compressive strength of HSSCC. Based on the Non Destructive Tests it is concluded that quality of concrete,
- Is Excellent for metakaolin 10% and 20%
- Is Good for metakaolin 30%.
- Is Medium for metakaolin 40%

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